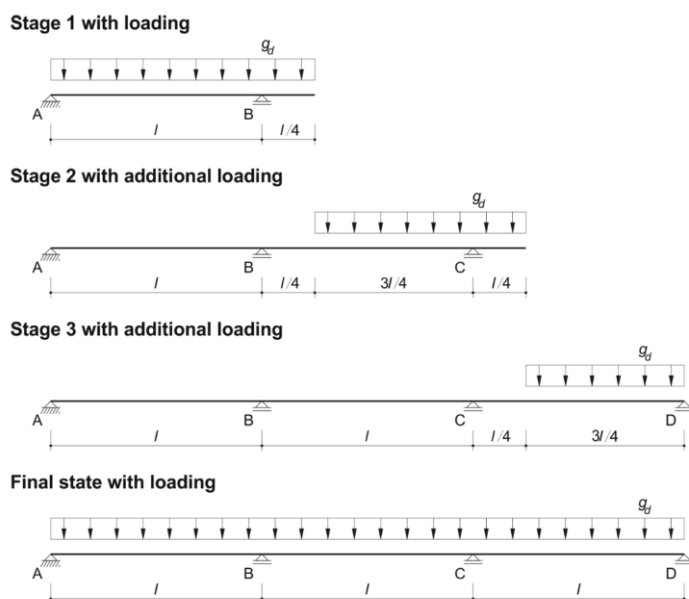


Constructing a three-span beam in stages

Geometry



Material Properties

Concrete	C35/45	$f_{ck} = 35 \text{ MPa}; f_{ctm} = 3.2 \text{ MPa}$ $f_{cd} = 22 \text{ MPa}; \tau_{cd} = 1.2 \text{ MPa}$ $E_{cm} = k_E \sqrt[3]{f_{cm}} \approx 35 \text{ GPa with } k_E = 10000$
Steel	B500B	$f_{sk} = 500 \text{ MPa}; f_{sd} = 435 \text{ MPa}$ $E_s = 205 \text{ GPa}$

SIA 262
Tab. 3

Tab. 8

3.1.2.3.3

Tab. 5/9

3.2.2.4

a) Creep coefficient $t_{120} = 120$ d, $t_{5y} = 5$ y

$$\varphi(t, t_0) = \varphi_{RH} \cdot \beta_{fc} \cdot \beta_{\sigma C} \cdot \beta(t_0) \cdot \beta(t - t_0)$$

with $\beta_{fc} = 2.6$, $\beta_{gc} = 1.0$

$$\varphi_{RH} = 1.25 \quad (RH = 70\%, \quad h_0 = 600 \text{ mm})$$

$$\beta(t_0) = 0.45 \quad (k_T = f(T = 20^\circ\text{C}) = 1.0, \text{ normal hardening cement})$$

3.1.2.6.2
3.1.2.6.3

Tab. 4
Task sheet
Fig. 2

- For $t = t_{120}$:

$$\beta(t-t_0) = \begin{Bmatrix} \beta(90 \text{ d}) & \text{St. 1} \\ \beta(60 \text{ d}) & \text{St. 2} \\ \beta(30 \text{ d}) & \text{St. 3} \end{Bmatrix} = \begin{Bmatrix} 0.42 \\ 0.38 \\ 0.32 \end{Bmatrix}$$

$$\varphi(t, t_0) = \left\{ \begin{array}{ll} \varphi(90 \text{ d}, 30 \text{ d}) & \text{St. 1} \\ \varphi(60 \text{ d}, 30 \text{ d}) & \text{St. 2} \\ \varphi(30 \text{ d}, 30 \text{ d}) & \text{St. 3} \end{array} \right\} = \left\{ \begin{array}{l} 0.614 \\ 0.556 \\ 0.468 \end{array} \right\}$$

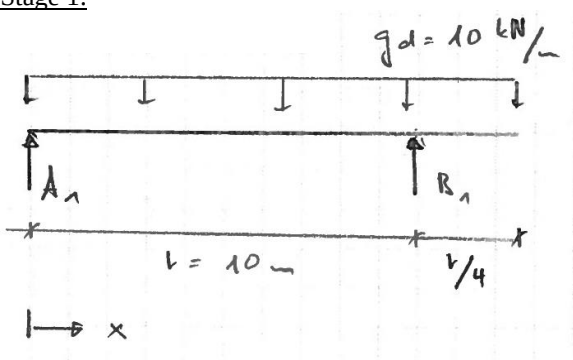
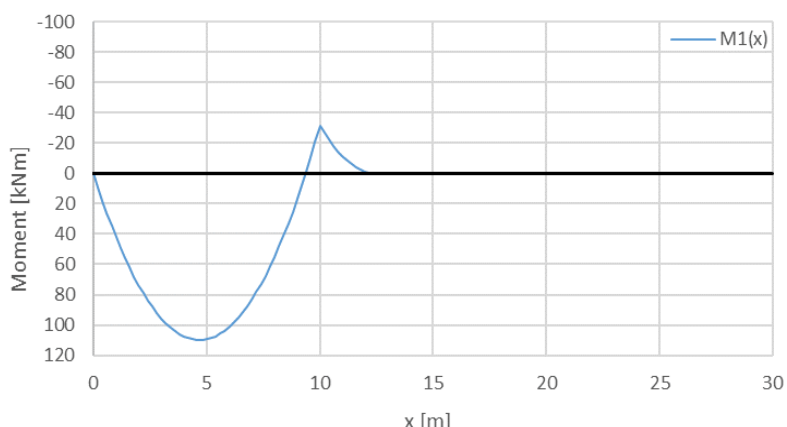
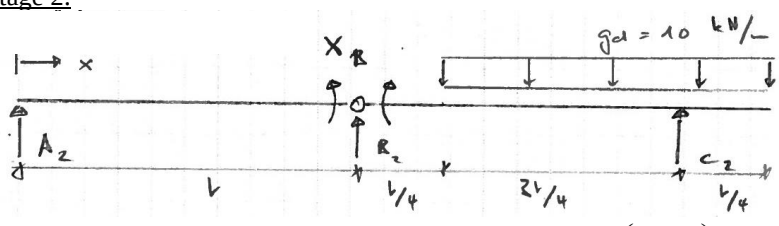
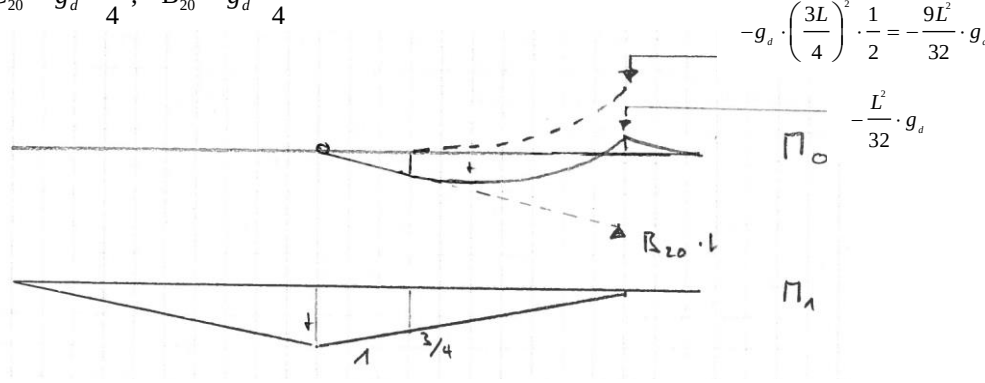
Fig. 2

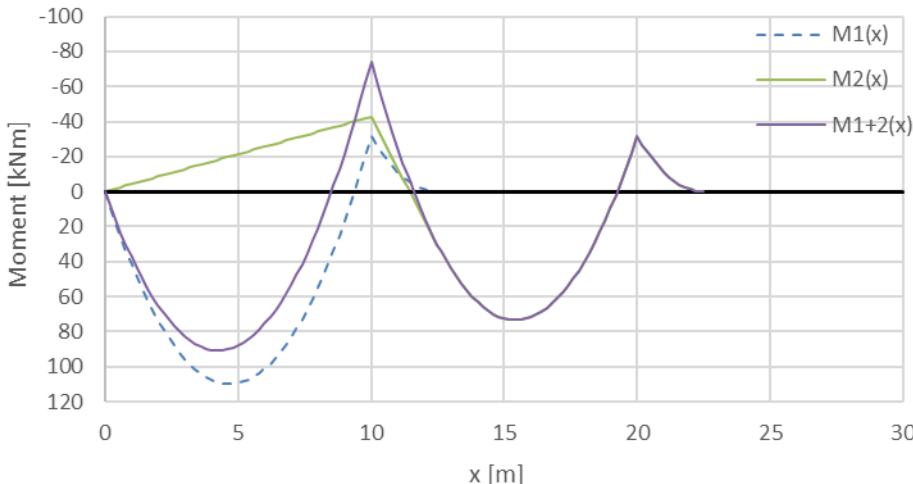
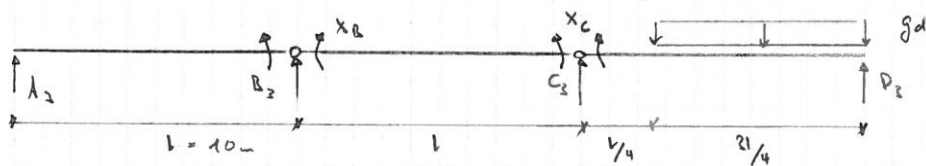
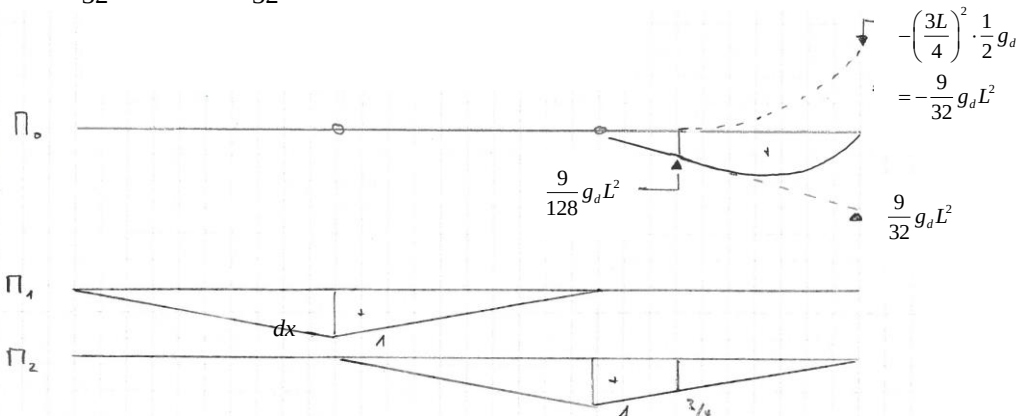
- For $t = t_{5v}$:

$$\beta(t-t_0) = 0.85 \quad (\text{approx. for all stages})$$

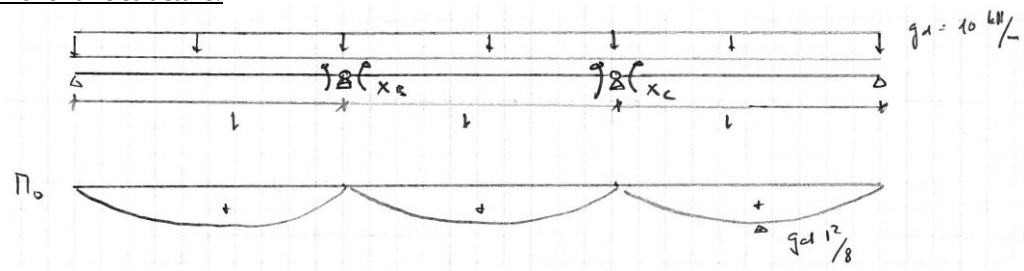
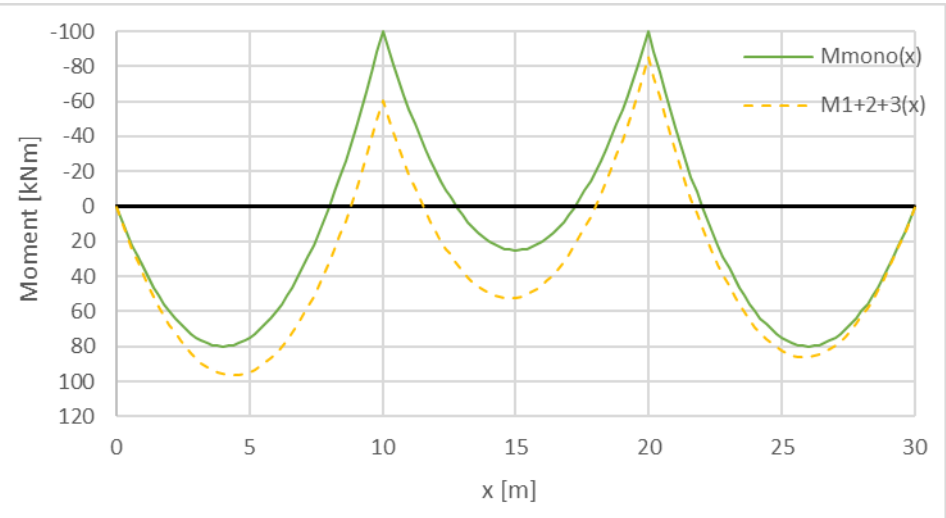
$$\varphi(t, t_0) = \varphi(5 \text{ y}, 30 \text{ d}) = 1.243$$

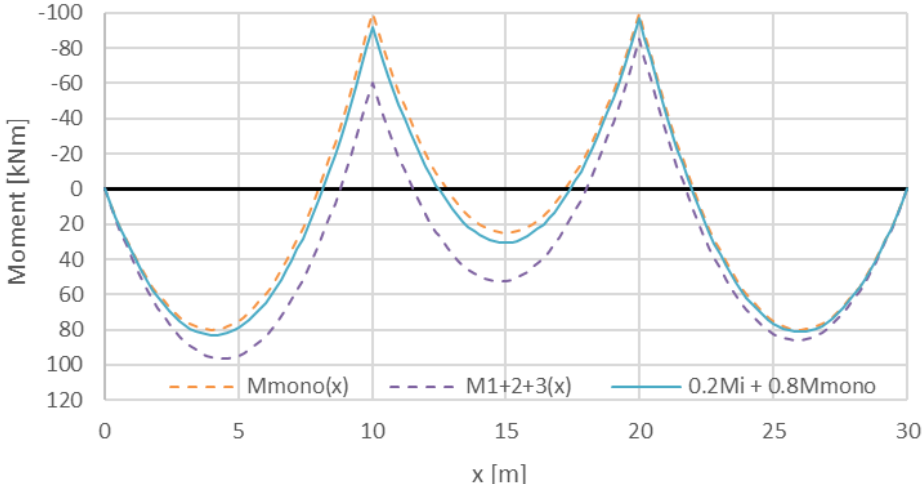
Fig.2

Advanced Structural Concrete		Page 2/7
Exercise 3	Solution	hs/lg/rev. yuk
b) <u>Moment distribution of each stage</u> <u>Stage 1:</u>  $\sum F_v = 0: A_1 + B_1 = g_d \cdot \frac{5L}{4}$ $\sum M_A = 0: g_d \cdot \left(\frac{5L}{4}\right)^2 \cdot \frac{1}{2} = B_1 \cdot L$ $\rightarrow B_1 = \frac{25}{32} \cdot g_d \cdot L, \quad A_1 = \frac{15}{32} \cdot g_d \cdot L$ $M_B = -g_d \cdot \left(\frac{L}{4}\right)^2 \cdot \frac{1}{2} = -g_d \cdot \frac{L^2}{32}$ $M_1 = \begin{cases} A_1 \cdot x - g_d \cdot \frac{x^2}{2} & \text{for } 0 \leq x \leq L \\ A_1 \cdot x + B_1 \cdot (x - L) - g_d \cdot \frac{x^2}{2} & \text{for } L < x \leq \frac{5L}{4} \end{cases}$  <u>Stage 2:</u>  $\sum F_v = 0: B_{20} + C_{20} = g_d \cdot L, \quad \sum M_B = 0: C_{20} \cdot L = g_d \cdot L \cdot \left(\frac{L}{4} + \frac{L}{2}\right)$ $\rightarrow C_{20} = g_d \cdot \frac{3L}{4}, \quad B_{20} = g_d \cdot \frac{L}{4}$  $-g_d \cdot \left(\frac{3L}{4}\right)^2 \cdot \frac{1}{2} = -\frac{9L^2}{32} \cdot g_d$ $-\frac{L^2}{32} \cdot g_d$ $B_{20} \cdot L$ M_0 M_1		Force method Basic system (BS) and redundant variables (RV) Dashed: moment curve, split for integration with force method

Advanced Structural Concrete		Page 3/7
Exercise 3	Solution	hs/lg/rev. yuk
$\delta_{10} = \frac{1}{6} \cdot \frac{L}{EI} \cdot B_{20} \cdot L \cdot 1 - \frac{1}{12} \cdot \frac{3L}{4EI} \cdot \left(\frac{9}{32} g_d L^2 \right) \cdot \frac{3}{4} = \frac{g_d \cdot L^3}{24EI} - \frac{27 \cdot g_d \cdot L^3}{2048EI}$$\delta_{11} = \frac{2L}{3EI}$$X_B = -\frac{\delta_{10}}{\delta_{11}} = -\frac{175}{4096} g_d \cdot L^2$$\rightarrow M_B = -42.725 \text{ kNm}, \quad M_C = -31.25 \text{ kNm}$$M_2(x) = \begin{cases} X_B \cdot \frac{x}{L} & 0 \leq x \leq L \\ X_B \cdot \left(1 - \frac{(x-L)}{L} \right) + B_{20}(x-L) & L < x \leq \frac{5L}{4} \\ X_B \cdot \left(1 - \frac{(x-L)}{L} \right) + B_{20}(x-L) - \frac{g_d}{2} \left(x - \frac{5L}{4} \right)^2 & \frac{5L}{4} < x \leq 2L \\ -\frac{g_d L^2}{32} + \frac{g_d L}{4} (x-2L) - \frac{g_d}{2} (x-2L)^2 & 2L < x \leq \frac{9L}{4} \end{cases}$		
Stage 3: $\sum F_V = 0: C_{30} + D_{30} = g_d \cdot \frac{3L}{4}, \quad \sum M_C = 0: D_{30} \cdot L = \frac{3L}{4} \cdot g_d \cdot \left(\frac{L}{4} + \frac{3L}{8} \right)$$\rightarrow D_{30} = \frac{15}{32} g_d L, \quad C_{30} = \frac{9}{32} g_d L$		
		Force method BS+RV
		Dashed: moment curve, split for integration with force method

Advanced Structural Concrete		Page 4/7
Exercise 3	Solution	hs/lg/rev. yuk
$\delta_{10} = 0$ $\delta_{20} = \frac{1}{6} \cdot \frac{L}{EI} \cdot 1 \cdot \frac{9L^2}{32} g_d - \frac{1}{12} \cdot \frac{3L}{4EI} \cdot \frac{9L^2}{32} g_d \cdot \frac{3}{4} = \frac{69}{2048} \frac{g_d L^3}{EI}$ $\delta_{11} = \delta_{22} = \frac{2}{3} \cdot \frac{L}{EI}$ $\delta_{12} = \delta_{21} = \frac{1}{6} \cdot \frac{L}{EI}$ $\rightarrow \begin{bmatrix} \delta_{10} \\ \delta_{20} \end{bmatrix} + \begin{bmatrix} \delta_{11} & \delta_{12} \\ \delta_{21} & \delta_{22} \end{bmatrix} \cdot \begin{bmatrix} X_B \\ X_C \end{bmatrix} = \underline{0}$ $\rightarrow \begin{bmatrix} 0 \\ \frac{69}{2048} \end{bmatrix} \cdot \frac{g_d L^3}{EI} + \begin{bmatrix} \frac{2}{3} & \frac{1}{6} \\ \frac{1}{6} & \frac{2}{3} \end{bmatrix} \cdot \frac{L}{EI} \cdot \begin{bmatrix} X_B \\ X_C \end{bmatrix} = \underline{0}$ $\rightarrow X_B = 0.013477 \cdot g_d L^2, \quad X_C = -\frac{207}{3840} g_d L^2 = -0.053906 \cdot g_d L^2$ $\rightarrow M_B = 13.477 \text{ kNm}, M_C = -53.906 \text{ kNm}$ $M_3(x) = \begin{cases} X_B \cdot \frac{x}{L} & 0 \leq x \leq L \\ X_B \cdot \left(2 - \frac{x}{L}\right) + X_C \frac{(x-L)}{L} & L < x \leq 2L \\ X_C \cdot \left(3 - \frac{x}{L}\right) + C_{30}(x-2L) & 2L < x \leq \frac{9L}{4} \\ X_C \cdot \left(3 - \frac{x}{L}\right) + C_{30}(x-2L) - g_d \frac{\left(x - \frac{9L}{4}\right)^2}{2} & \frac{9L}{4} < x \leq 3L \end{cases}$ 		

Advanced Structural Concrete		Page 5/7
Exercise 3	Solution	hs/lg/rev. yuk
<u>Monolithic structure:</u>  M_1 and M_2 analogues to Stage 3. From symmetry: $X_B = X_C$. $\delta_{10} = \delta_{20} = \frac{1}{3} \cdot \frac{L}{EI} \cdot \frac{q_d L^2}{8} \cdot 1 \cdot 2 = \frac{1}{12} \cdot \frac{q_d L^3}{EI}$ $\delta_{11} = \delta_{22} = \frac{2}{3} \cdot \frac{L}{EI}, \quad \delta_{12} = \delta_{21} = \frac{1}{6} \cdot \frac{L}{EI} \quad (\text{from stage 3})$ $\rightarrow \delta_1 = \delta_{10} + \delta_{11} X_B + \delta_{12} X_C \rightarrow X_B = -\frac{q_d L^2}{10} = X_C$ $M_{Mono}(x) = \begin{cases} \frac{q_d L}{2} \cdot x - q_d \frac{x^2}{2} - \frac{q_d L^2}{10} \frac{x}{L} & 0 \leq x \leq L \\ \frac{q_d L}{2} \cdot (x-L) - q_d \frac{(x-L)^2}{2} - \frac{q_d L^2}{10} & L < x \leq 2L \\ \frac{q_d L}{2} \cdot (x-2L) - q_d \frac{(x-2L)^2}{2} - \frac{q_d L^2}{10} \left(1 - \frac{x-2L}{L}\right) & 2L < x \leq 3L \end{cases}$ 		Force method BS+RV

Advanced Structural Concrete		Page 6/7
Exercise 3	Solution	hs/lg/rev. yuk
c) <u>Approximation of the moment distribution for $t \rightarrow \infty$</u> $M_{t \rightarrow \infty}(x) = 0.2 \cdot (M_1(x) + M_2(x) + M_3(x)) + 0.8 \cdot M_{Mono}(x)$ 		
d) <u>Moment distribution with the Trost Method</u> $M_t(x) = M_0(x) + (M_{Mono}(x) - M_0(x)) \cdot \frac{\varphi(t, t_0)}{1 + \mu \cdot \varphi(t, t_0)}$ $M_t(x) = M_0(x) \cdot \left(1 - \frac{\varphi(t, t_0)}{1 + \mu \cdot \varphi(t, t_0)}\right) + M_{Mono}(x) \cdot \frac{\varphi(t, t_0)}{1 + \mu \cdot \varphi(t, t_0)}$ M_0 : Moment before system change M_{Mono} : Moment of monolithic system Generalized: $M_t(x) = \sum_{i=1}^n M_{0,i}(x) \cdot \left(1 - \frac{\varphi(t_i, t_0)}{1 + \mu \cdot \varphi(t_i, t_0)}\right) + M_{Mono}(x) \cdot \frac{\varphi(t, t_0)}{1 + \mu \cdot \varphi(t, t_0)}$ $M_{120d}(x) = M_1(x) \cdot \left(1 - \frac{\varphi(90d, 30d)}{1 + \mu \cdot \varphi(90d, 30d)}\right) + M_2(x) \cdot \left(1 - \frac{\varphi(60d, 30d)}{1 + \mu \cdot \varphi(60d, 30d)}\right) + M_3(x) \cdot \left(1 - \frac{\varphi(30d, 30d)}{1 + \mu \cdot \varphi(30d, 30d)}\right) + M_{Mono}(x) \cdot \frac{\varphi(90d, 30d)}{1 + \mu \cdot \varphi(90d, 30d)}$ $M_{120d}(x) = 0.588M_1(x) + 0.615M_2(x) + 0.659M_3(x) + 0.412M_{Mono}(x)$ $M_{5y}(x) = (M_1(x) + M_2(x) + M_3(x)) \cdot \left(1 - \frac{\varphi(5y, 30d)}{1 + \mu \cdot \varphi(5y, 30d)}\right) + M_{Mono}(x) \cdot \frac{\varphi(5y, 30d)}{1 + \mu \cdot \varphi(5y, 30d)}$ $M_{5y}(x) = 0.377 \cdot (M_1(x) + M_2(x) + M_3(x)) + 0.623M_{Mono}(x)$		: $\mu = 0.8$

Advanced Structural Concrete		Page 7/7
Exercise 3	Solution	hs/lg/rev. yuk
Remark: As the age of the structure increases, the distribution of moments increasingly approaches that of the monolithic structure due to creep. The approximation "80% monolithic structure, 20% sum of the individual construction stages" approximates the long-term behaviour relatively well. Since the determination of the creep coefficient is also subject to certain uncertainty and the bending stiffness over the beam length is by no means constant (crack formation), the calculation with the Trost method can only be regarded as an approximation of the real stress state.		